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REFERENCE: N/A

PROJECT: BP8.R026

STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	BP8.R026	1	

STATE OF NORTH CAROLINA
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

STRUCTURE SUBSURFACE INVESTIGATION

COUNTY RANDOLPH

PROJECT DESCRIPTION BRIDGE 750176 ON SR 2207 (FAITH ROCK RD) OVER UNNAMED CREEK

INVENTORY

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CAUTION NOTICE

THE SUBSURFACE INFORMATION AND THE SUBSURFACE INVESTIGATION ON WHICH IT IS BASED WERE MADE FOR THE PURPOSE OF STUDY, PLANNING AND DESIGN, AND NOT FOR CONSTRUCTION OR PAY PURPOSES. THE VARIOUS FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA AVAILABLE MAY BE REVIEWED OR INSPECTED IN RALEIGH BY CONTACTING THE N. C. DEPARTMENT OF TRANSPORTATION, GEOTECHNICAL ENGINEERING UNIT AT (919) 707-6850. THE SUBSURFACE PLANS AND REPORTS, FIELD BORING LOGS, ROCK CORES AND SOIL TEST DATA ARE NOT PART OF THE CONTRACT.

GENERAL SOIL AND ROCK STRATA DESCRIPTIONS AND INDICATED BOUNDARIES ARE BASED ON A GEOTECHNICAL INTERPRETATION OF ALL AVAILABLE SUBSURFACE DATA AND MAY NOT NECESSARILY REFLECT THE ACTUAL SUBSURFACE CONDITIONS BETWEEN BORINGS OR BETWEEN SAMPLED STRATA WITHIN THE BOREHOLE. THE LABORATORY SAMPLE DATA AND THE IN SITU (IN-PLACE) TEST DATA CAN BE RELIED ON ONLY TO THE DEGREE OF RELIABILITY INHERENT IN THE STANDARD TEST METHOD. THE OBSERVED WATER LEVELS OR SOIL MOISTURE CONDITIONS INDICATED IN THE SUBSURFACE INVESTIGATIONS ARE AS RECORDED AT THE TIME OF THE INVESTIGATION. THESE WATER LEVELS OR SOIL MOISTURE CONDITIONS MAY VARY CONSIDERABLY WITH TIME ACCORDING TO CLIMATIC CONDITIONS INCLUDING TEMPERATURES, PRECIPITATION AND WIND, AS WELL AS OTHER NON-CLIMATIC FACTORS.

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NOTES:
1. THE INFORMATION CONTAINED HEREIN IS NOT IMPLIED OR GUARANTEED BY THE N. C. DEPARTMENT OF TRANSPORTATION AS ACCURATE NOR IS IT CONSIDERED PART OF THE PLANS, SPECIFICATIONS OR CONTRACT FOR THE PROJECT.
2. BY HAVING REQUESTED THIS INFORMATION, THE CONTRACTOR SPECIFICALLY WAIVES ANY CLAIMS FOR INCREASED COMPENSATION OR EXTENSION OF TIME BASED ON DIFFERENCES BETWEEN THE CONDITIONS INDICATED HEREIN AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

PERSONNEL

C.T. TANG

J. HOWARD

M. MOSELEY

A. GROSS

INVESTIGATED BY WSP USA

DRAWN BY C.T. TANG

CHECKED BY S. WOODS

SUBMITTED BY C.T.TANG

DATE FEBRUARY 2024

NC Engineering F-0165 NC Geology C-557

NORTH CAROLINA PROFESSIONAL SEAL 047389 ENGINEER CHIEN-TING TANG

DocuSigned by:
Chien-Ting Tang
4071910370EE41F...

2/18/2025

SIGNATURE DATE

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BP.8.R026	2

NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SOIL AND ROCK LEGEND, TERMS, SYMBOLS, AND ABBREVIATIONS

SOIL DESCRIPTION

SOIL IS CONSIDERED UNCONSOLIDATED, SEMI-CONSOLIDATED, OR WEATHERED EARTH MATERIALS THAT CAN BE PENETRATED WITH A CONTINUOUS FLIGHT POWER AUGER AND YIELD LESS THAN 100 BLOWS PER FOOT ACCORDING TO THE STANDARD PENETRATION TEST (AASHTO T 206, ASTM D1586). SOIL CLASSIFICATION IS BASED ON THE AASHTO SYSTEM. BASIC DESCRIPTIONS GENERALLY INCLUDE THE FOLLOWING: CONSISTENCY, COLOR, TEXTURE, MOISTURE, AASHTO CLASSIFICATION, AND OTHER PERTINENT FACTORS SUCH AS MINERALOGICAL COMPOSITION, ANGULARITY, STRUCTURE, PLASTICITY, ETC. FOR EXAMPLE,
VERY STIFF, GRAY, SILTY CLAY, MOIST WITH INTERBEDDED FINE SAND LAYERS, HIGHLY PLASTIC, A-7-6

SOIL LEGEND AND AASHTO CLASSIFICATION

GENERAL CLASS.	GRANULAR MATERIALS (≤ 35% PASSING #200)							SILT-CLAY MATERIALS (> 35% PASSING #200)				ORGANIC MATERIALS		
GROUP CLASS.	A-1		A-3	A-2				A-4	A-5	A-6	A-7	A-1, A-2 A-3	A-4, A-5 A-6, A-7	
	A-1-a	A-1-b		A-2-4	A-2-5	A-2-6	A-2-7				A-7-5, A-7-6			
SYMBOL														
% PASSING #10 #40 #200	50 MX 30 MX 15 MX	50 MX 25 MX	51 MN 10 MX	35 MX	35 MX	35 MX	35 MX	36 MN	36 MN	36 MN	36 MN	GRANULAR SOILS	SILT-CLAY SOILS	MUCK, PEAT
MATERIAL PASSING #40 LL PI	— 6 MX		— NP	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	40 MX 10 MX	41 MN 10 MX	40 MX 11 MN	41 MN 11 MN	SOILS WITH LITTLE OR MODERATE AMOUNTS OF ORGANIC MATTER		HIGHLY ORGANIC SOILS
GROUP INDEX	0		0	0		4 MX		8 MX	12 MX	16 MX	NO MX			
USUAL TYPES OF MAJOR MATERIALS	STONE FRAGS. GRAVEL, AND SAND		FINE SAND	SILTY OR CLAYEY GRAVEL AND SAND				SILTY SOILS		CLAYEY SOILS				
GEN. RATING AS SUBGRADE	EXCELLENT TO GOOD							FAIR TO POOR				FAIR TO POOR	POOR	UNSUITABLE

PI OF A-7-5 SUBGROUP IS ≤ LL - 30 ; PI OF A-7-6 SUBGROUP IS > LL - 30

CONSISTENCY OR DENSENESS

PRIMARY SOIL TYPE	COMPACTNESS OR CONSISTENCY	RANGE OF STANDARD PENETRATION RESISTENCE (N-VALUE)	RANGE OF UNCONFINED COMPRESSIVE STRENGTH ($TONS/FT^2$)
GENERALLY GRANULAR MATERIAL (NON-COHESIVE)	VERY LOOSE LOOSE MEDIUM DENSE DENSE VERY DENSE	< 4 4 TO 10 10 TO 30 30 TO 50 > 50	N/A
GENERALLY SILT-CLAY MATERIAL (COHESIVE)	VERY SOFT SOFT MEDIUM STIFF STIFF VERY STIFF HARD	< 2 2 TO 4 4 TO 8 8 TO 15 15 TO 30 > 30	< 0.25 0.25 TO 0.5 0.5 TO 1.0 1 TO 2 2 TO 4 > 4

TEXTURE OR GRAIN SIZE

U.S. STD. SIEVE SIZE	4	10	40	60	200	270
OPENING (MM)	4.76	2.00	0.42	0.25	0.075	0.053

BOULDER (BLDR.)	COBBLE (COB.)	GRAVEL (GR.)	COARSE SAND (CSE. SD.)	FINE SAND (F SD.)	SILT (SL.)	CLAY (CL.)

GRAIN SIZE	MM	305	75	2.0	0.25	0.05	0.005
	IN.	12	3				

SOIL MOISTURE - CORRELATION OF TERMS

SOIL MOISTURE SCALE (ATTERBERG LIMITS)	FIELD MOISTURE DESCRIPTION	GUIDE FOR FIELD MOISTURE DESCRIPTION
	- SATURATED - (SAT.)	USUALLY LIQUID; VERY WET, USUALLY FROM BELOW THE GROUND WATER TABLE
LL — LIQUID LIMIT		
PLASTIC RANGE (PI)	- WET - (W)	SEMISOLID; REQUIRES DRYING TO ATTAIN OPTIMUM MOISTURE
PL — PLASTIC LIMIT		
OM — OPTIMUM MOISTURE SL — SHRINKAGE LIMIT	- MOIST - (M)	SOLID; AT OR NEAR OPTIMUM MOISTURE
	- DRY - (D)	REQUIRES ADDITIONAL WATER TO ATTAIN OPTIMUM MOISTURE

PLASTICITY

	PLASTICITY INDEX (PI)	DRY STRENGTH
NON PLASTIC	0-5	VERY LOW
SLIGHTLY PLASTIC	6-15	SLIGHT
MODERATELY PLASTIC	16-25	MEDIUM
HIGHLY PLASTIC	26 OR MORE	HIGH

COLOR

DESCRIPTIONS MAY INCLUDE COLOR OR COLOR COMBINATIONS (TAN, RED, YELLOW-BROWN, BLUE-GRAY), MODIFIERS SUCH AS LIGHT, DARK, STREAKED, ETC. ARE USED TO DESCRIBE APPEARANCE.

GRADATION

WELL GRADED - INDICATES A GOOD REPRESENTATION OF PARTICLE SIZES FROM FINE TO COARSE.
UNIFORMLY GRADED - INDICATES THAT SOIL PARTICLES ARE ALL APPROXIMATELY THE SAME SIZE.
GAP-GRADED - INDICATES A MIXTURE OF UNIFORM PARTICLE SIZES OF TWO OR MORE SIZES.

ANGULARITY OF GRAINS

THE ANGULARITY OR ROUNDNESS OF SOIL GRAINS IS DESIGNATED BY THE TERMS: ANGULAR, SUBANGULAR, SUBROUNDED, OR ROUNDED.

MINERALOGICAL COMPOSITION

MINERAL NAMES SUCH AS QUARTZ, FELDSPAR, MICA, TALC, KAOLIN, ETC. ARE USED IN DESCRIPTIONS WHEN THEY ARE CONSIDERED OF SIGNIFICANCE.

COMPRESSIBILITY

SLIGHTLY COMPRESSIBLE	LL < 31
MODERATELY COMPRESSIBLE	LL = 31 - 50
HIGHLY COMPRESSIBLE	LL > 50

PERCENTAGE OF MATERIAL

ORGANIC MATERIAL	GRANULAR SOILS	SILT - CLAY SOILS	OTHER MATERIAL	
TRACE OF ORGANIC MATTER	2 - 3%	3 - 5%	TRACE	1 - 10%
LITTLE ORGANIC MATTER	3 - 5%	5 - 12%	LITTLE	10 - 20%
MODERATELY ORGANIC	5 - 10%	12 - 20%	SOME	20 - 35%
HIGHLY ORGANIC	> 10%	> 20%	HIGHLY	35% AND ABOVE

GROUND WATER

- WATER LEVEL IN BORE HOLE IMMEDIATELY AFTER DRILLING
- STATIC WATER LEVEL AFTER 24 HOURS
- PW — PERCHED WATER, SATURATED ZONE, OR WATER BEARING STRATA
- SPRING OR SEEP

MISCELLANEOUS SYMBOLS

- ROADWAY EMBANKMENT (RE) WITH SOIL DESCRIPTION
- SOIL SYMBOL
- ARTIFICIAL FILL (AF) OTHER THAN ROADWAY EMBANKMENT
- INFERRED SOIL BOUNDARY
- INFERRED ROCK LINE
- ALLUVIAL SOIL BOUNDARY
- 25/025 — DIP & DIP DIRECTION OF ROCK STRUCTURES
- SPT, DPT, DMT, VST, PMT — TEST BORING
- AUGER BORING
- CORE BORING
- MW — MONITORING WELL
- PIEZOMETER INSTALLATION
- SLOPE INDICATOR INSTALLATION
- CONE PENETROMETER TEST
- SOUNDING ROD
- TEST BORING WITH CORE
- SPT N-VALUE

RECOMMENDATION SYMBOLS

- UNDERCUT
- SHALLOW UNDERCUT
- UNCLASSIFIED EXCAVATION - UNSUITABLE WASTE
- UNCLASSIFIED EXCAVATION - ACCEPTABLE DEGRADABLE ROCK
- UNCLASSIFIED EXCAVATION - ACCEPTABLE, BUT NOT TO BE USED IN THE TOP 3 FEET OF EMBANKMENT OR BACKFILL

ABBREVIATIONS

AR - AUGER REFUSAL
BT - BORING TERMINATED
CL. - CLAY
CPT - CONE PENETRATION TEST
CSE. - COARSE
DMT - DILATOMETER TEST
DPT - DYNAMIC PENETRATION TEST
e - VOID RATIO
F - FINE
FOSS. - FOSSILIFEROUS
FRAC. - FRACTURED, FRACTURES
FRAGS. - FRAGMENTS
HI. - HIGHLY
MED. - MEDIUM
MICA. - MICACEOUS
MOD. - MODERATELY
NP - NON PLASTIC
ORG. - ORGANIC
PMT - PRESSUREMETER TEST
SAP. - SAPROLITIC
SD. - SAND, SANDY
SL. - SILT, SILTY
SLI. - SLIGHTLY
TCR - TRICONE REFUSAL
w - MOISTURE CONTENT
V - VERY
VST - VANE SHEAR TEST
WEA. - WEATHERED
γ - UNIT WEIGHT
γ_d - DRY UNIT WEIGHT

SAMPLE ABBREVIATIONS

S - BULK
SS - SPLIT SPOON
ST - SHELBY TUBE
RS - ROCK
RT - RECOMPACTED TRIAXIAL
CBR - CALIFORNIA BEARING RATIO

EQUIPMENT USED ON SUBJECT PROJECT

DRILL UNITS:
- [X] CME-45C
- [ ] CME-55
- [ ] CME-550
- [ ] VANE SHEAR TEST
- [ ] PORTABLE HOIST
- [ ] _____
- [ ] _____

ADVANCING TOOLS:
- [ ] CLAY BITS
- [ ] 6" CONTINUOUS FLIGHT AUGER
- [X] 8" HOLLOW AUGERS
- [ ] HARD FACED FINGER BITS
- [ ] TUNG.-CARBIDE INSERTS
- [X] CASING [ ] W/ ADVANCER
- [ ] TRICONE _____" STEEL TEETH
- [ ] TRICONE _____" TUNG.-CARB.
- [ ] CORE BIT
- [ ] _____

HAMMER TYPE:
- [X] AUTOMATIC [ ] MANUAL

CORE SIZE:
- [ ] -B _____ [ ] -H _____
- [X] -N Q

HAND TOOLS:
- [ ] POST HOLE DIGGER
- [ ] HAND AUGER
- [ ] SOUNDING ROD
- [ ] VANE SHEAR TEST
- [ ] _____

ROCK DESCRIPTION

HARD ROCK IS NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT REFUSAL IF TESTED. AN INFERRED ROCK LINE INDICATES THE LEVEL AT WHICH NON-COASTAL PLAIN MATERIAL WOULD YIELD SPT REFUSAL. SPT REFUSAL IS PENETRATION BY A SPLIT SPOON SAMPLER EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS IN NON-COASTAL PLAIN MATERIAL. THE TRANSITION BETWEEN SOIL AND ROCK IS OFTEN REPRESENTED BY A ZONE OF WEATHERED ROCK.
ROCK MATERIALS ARE TYPICALLY DIVIDED AS FOLLOWS:

WEATHERED ROCK (WR)	NON-COASTAL PLAIN MATERIAL THAT WOULD YIELD SPT N VALUES > 100 BLOWS PER FOOT IF TESTED.
CRYSTALLINE ROCK (CR)	FINE TO COARSE GRAIN IGNEOUS AND METAMORPHIC ROCK THAT WOULD YIELD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES GRANITE, GNEISS, GABBRO, SCHIST, ETC.
NON-CRYSTALLINE ROCK (NCR)	FINE TO COARSE GRAIN METAMORPHIC AND NON-COASTAL PLAIN SEDIMENTARY ROCK THAT WOULD YEILD SPT REFUSAL IF TESTED. ROCK TYPE INCLUDES PHYLLITE, SLATE, SANDSTONE, ETC.
COASTAL PLAIN SEDIMENTARY ROCK (CP)	COASTAL PLAIN SEDIMENTS CEMENTED INTO ROCK, BUT MAY NOT YIELD SPT REFUSAL. ROCK TYPE INCLUDES LIMESTONE, SANDSTONE, CEMENTED SHELL BEDS, ETC.

WEATHERING

FRESH	ROCK FRESH, CRYSTALS BRIGHT, FEW JOINTS MAY SHOW SLIGHT STAINING. ROCK RINGS UNDER HAMMER IF CRYSTALLINE.
VERY SLIGHT (V SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED, SOME JOINTS MAY SHOW THIN CLAY COATINGS IF OPEN, CRYSTALS ON A BROKEN SPECIMEN FACE SHINE BRIGHTLY. ROCK RINGS UNDER HAMMER BLOWS IF OF A CRYSTALLINE NATURE.
SLIGHT (SLI.)	ROCK GENERALLY FRESH, JOINTS STAINED AND DISCOLORATION EXTENDS INTO ROCK UP TO 1 INCH. OPEN JOINTS MAY CONTAIN CLAY. IN GRANITOID ROCKS SOME OCCASIONAL FELDSPAR CRYSTALS ARE DULL AND DISCOLORED. CRYSTALLINE ROCKS RING UNDER HAMMER BLOWS.
MODERATE (MOD.)	SIGNIFICANT PORTIONS OF ROCK SHOW DISCOLORATION AND WEATHERING EFFECTS. IN GRANITOID ROCKS, MOST FELDSPARS ARE DULL AND DISCOLORED, SOME SHOW CLAY. ROCK HAS DULL SOUND UNDER HAMMER BLOWS AND SHOWS SIGNIFICANT LOSS OF STRENGTH AS COMPARED WITH FRESH ROCK.
MODERATELY SEVERE (MOD. SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. IN GRANITOID ROCKS, ALL FELDSPARS DULL AND DISCOLORED AND A MAJORITY SHOW KAOLINIZATION. ROCK SHOWS SEVERE LOSS OF STRENGTH AND CAN BE EXCAVATED WITH A GEOLOGIST'S PICK. ROCK GIVES "CLUNK" SOUND WHEN STRUCK. *IF TESTED, WOULD YIELD SPT REFUSAL*
SEVERE (SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC CLEAR AND EVIDENT BUT REDUCED IN STRENGTH TO STRONG SOIL. IN GRANITOID ROCKS ALL FELDSPARS ARE KAOLINIZED TO SOME EXTENT. SOME FRAGMENTS OF STRONG ROCK USUALLY REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES > 100 BPF*
VERY SEVERE (V SEV.)	ALL ROCK EXCEPT QUARTZ DISCOLORED OR STAINED. ROCK FABRIC ELEMENTS ARE DISCERNIBLE BUT MASS IS EFFECTIVELY REDUCED TO SOIL STATUS, WITH ONLY FRAGMENTS OF STRONG ROCK REMAINING. SAPROLITE IS AN EXAMPLE OF ROCK WEATHERED TO A DEGREE THAT ONLY MINOR VESTIGES OF ORIGINAL ROCK FABRIC REMAIN. *IF TESTED, WOULD YIELD SPT N VALUES < 100 BPF*
COMPLETE	ROCK REDUCED TO SOIL. ROCK FABRIC NOT DISCERNIBLE, OR DISCERNIBLE ONLY IN SMALL AND SCATTERED CONCENTRATIONS. QUARTZ MAY BE PRESENT AS DIKES OR STRINGERS. SAPROLITE IS ALSO AN EXAMPLE.

ROCK HARDNESS

VERY HARD	CANNOT BE SCRATCHED BY KNIFE OR SHARP PICK. BREAKING OF HAND SPECIMENS REQUIRES SEVERAL HARD BLOWS OF THE GEOLOGIST'S PICK.
HARD	CAN BE SCRATCHED BY KNIFE OR PICK ONLY WITH DIFFICULTY. HARD HAMMER BLOWS REQUIRED TO DETACH HAND SPECIMEN.
MODERATELY HARD	CAN BE SCRATCHED BY KNIFE OR PICK. GOUGES OR GROOVES TO 0.25 INCHES DEEP CAN BE EXCAVATED BY HARD BLOW OF A GEOLOGIST'S PICK. HAND SPECIMENS CAN BE DETACHED BY MODERATE BLOWS.
MEDIUM HARD	CAN BE GROOVED OR GOUGED 0.05 INCHES DEEP BY FIRM PRESSURE OF KNIFE OR PICK POINT. CAN BE EXCAVATED IN SMALL CHIPS TO PEICES 1 INCH MAXIMUM SIZE BY HARD BLOWS OF THE POINT OF A GEOLOGIST'S PICK.
SOFT	CAN BE GROVED OR GOUGED READILY BY KNIFE OR PICK. CAN BE EXCAVATED IN FRAGMENTS FROM CHIPS TO SEVERAL INCHES IN SIZE BY MODERATE BLOWS OF A PICK POINT. SMALL, THIN PIECES CAN BE BROKEN BY FINGER PRESSURE.
VERY SOFT	CAN BE CARVED WITH KNIFE. CAN BE EXCAVATED READILY WITH POINT OF PICK. PIECES 1 INCH OR MORE IN THICKNESS CAN BE BROKEN BY FINGER PRESSURE. CAN BE SCRATCHED READILY BY FINGERNAIL.

FRACTURE SPACING

TERM	SPACING
VERY WIDE	MORE THAN 10 FEET
WIDE	3 TO 10 FEET
MODERATELY CLOSE	1 TO 3 FEET
CLOSE	0.16 TO 1 FOOT
VERY CLOSE	LESS THAN 0.16 FEET

BEDDING

TERM	THICKNESS
VERY THICKLY BEDDED	4 FEET
THICKLY BEDDED	1.5 - 4 FEET
THINLY BEDDED	0.16 - 1.5 FEET
VERY THINLY BEDDED	0.03 - 0.16 FEET
THICKLY LAMINATED	0.008 - 0.03 FEET
THINLY LAMINATED	< 0.008 FEET

INDURATION

FOR SEDIMENTARY ROCKS, INDURATION IS THE HARDENING OF MATERIAL BY CEMENTING, HEAT, PRESSURE, ETC.

FRIABLE	RUBBING WITH FINGER FREES NUMEROUS GRAINS; GENTLE BLOW BY HAMMER DISINTEGRATES SAMPLE.
MODERATELY INDURATED	GRAINS CAN BE SEPARATED FROM SAMPLE WITH STEEL PROBE; BREAKS EASILY WHEN HIT WITH HAMMER.
INDURATED	GRAINS ARE DIFFICULT TO SEPARATE WITH STEEL PROBE; DIFFICULT TO BREAK WITH HAMMER.
EXTREMELY INDURATED	SHARP HAMMER BLOWS REQUIRED TO BREAK SAMPLE; SAMPLE BREAKS ACROSS GRAINS.

TERMS AND DEFINITIONS

ALLUVIUM (ALLUV.) - SOILS THAT HAVE BEEN TRANSPORTED BY WATER.

AQUIFER - A WATER BEARING FORMATION OR STRATA.

ARENACEOUS - APPLIED TO ROCKS THAT HAVE BEEN DERIVED FROM SAND OR THAT CONTAIN SAND.

ARGILLACEOUS - APPLIED TO ALL ROCKS OR SUBSTANCES COMPOSED OF CLAY MINERALS, OR HAVING A NOTABLE PROPORTION OF CLAY IN THEIR COMPOSITION, SUCH AS SHALE, SLATE, ETC.

ARTESIAN - GROUND WATER THAT IS UNDER SUFFICIENT PRESSURE TO RISE ABOVE THE LEVEL AT WHICH IT IS ENCOUNTERED, BUT WHICH DOES NOT NECESSARILY RISE TO OR ABOVE THE GROUND SURFACE.

CALCAREOUS (CALC.) - SOILS THAT CONTAIN APPRECIABLE AMOUNTS OF CALCIUM CARBONATE.

COLLUVIUM - ROCK FRAGMENTS MIXED WITH SOIL DEPOSITED BY GRAVITY ON SLOPE OR AT BOTTOM OF SLOPE.

CORE RECOVERY (REC.) - TOTAL LENGTH OF ALL MATERIAL RECOVERED IN THE CORE BARREL DIVIDED BY TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

DIKE - A TABULAR BODY OF IGNEOUS ROCK THAT CUTS ACROSS THE STRUCTURE OF ADJACENT ROCKS OR CUTS MASSIVE ROCK.

DIP - THE ANGLE AT WHICH A STRATUM OR ANY PLANAR FEATURE IS INCLINED FROM THE HORIZONTAL.

DIP DIRECTION (DIP AZIMUTH) - THE DIRECTION OR BEARING OF THE HORIZONTAL TRACE OF THE LINE OF DIP, MEASURED CLOCKWISE FROM NORTH.

FAULT - A FRACTURE OR FRACTURE ZONE ALONG WHICH THERE HAS BEEN DISPLACEMENT OF THE SIDES RELATIVE TO ONE ANOTHER PARALLEL TO THE FRACTURE.

FISSILE - A PROPERTY OF SPLITTING ALONG CLOSELY SPACED PARALLEL PLANES.

FLOAT - ROCK FRAGMENTS ON SURFACE NEAR THEIR ORIGINAL POSITION AND DISLODGED FROM PARENT MATERIAL.

FLOOD PLAIN (FP) - LAND BORDERING A STREAM, BUILT OF SEDIMENTS DEPOSITED BY THE STREAM.

FORMATION (FM.) - A MAPPABLE GEOLOGIC UNIT THAT CAN BE RECOGNIZED AND TRACED IN THE FIELD.

JOINT - FRACTURE IN ROCK ALONG WHICH NO APPRECIABLE MOVEMENT HAS OCCURRED.

LEDGE - A SHELF-LIKE RIDGE OR PROJECTION OF ROCK WHOSE THICKNESS IS SMALL COMPARED TO ITS LATERAL EXTENT.

LENS - A BODY OF SOIL OR ROCK THAT THINS OUT IN ONE OR MORE DIRECTIONS.

MOTTLED (MOT.) - IRREGULARLY MARKED WITH SPOTS OF DIFFERENT COLORS. MOTTLING IN SOILS USUALLY INDICATES POOR AERATION AND LACK OF GOOD DRAINAGE.

PERCHED WATER - WATER MAINTAINED ABOVE THE NORMAL GROUND WATER LEVEL BY THE PRESENCE OF AN INTERVENING IMPERVIOUS STRATUM.

RESIDUAL (RES.) SOIL - SOIL FORMED IN PLACE BY THE WEATHERING OF ROCK.

ROCK QUALITY DESIGNATION (RQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF CORE RUN AND EXPRESSED AS A PERCENTAGE.

SAPROLITE (SAP.) - RESIDUAL SOIL THAT RETAINS THE RELIC STRUCTURE OR FABRIC OF THE PARENT ROCK.

SILL - AN INTRUSIVE BODY OF IGNEOUS ROCK OF APPROXIMATELY UNIFORM THICKNESS AND RELATIVELY THIN COMPARED WITH ITS LATERAL EXTENT, THAT HAS BEEN EMPLACED PARALLEL TO THE BEDDING OR SCHISTOSITY OF THE INTRUDED ROCKS.

SLICKENSIDE - POLISHED AND STRIATED SURFACE THAT RESULTS FROM FRICTION ALONG A FAULT OR SLIP PLANE.

STANDARD PENETRATION TEST (PENETRATION RESISTANCE) (SPT) - NUMBER OF BLOWS (N OR BPF) OF A 140 LB. HAMMER FALLING 30 INCHES REQUIRED TO PRODUCE A PENETRATION OF 1 FOOT INTO SOIL WITH A 2 INCH OUTSIDE DIAMETER SPLIT SPOON SAMPLER. SPT REFUSAL IS PENETRATION EQUAL TO OR LESS THAN 0.1 FOOT PER 60 BLOWS.

STRATA CORE RECOVERY (SREC.) - TOTAL LENGTH OF STRATA MATERIAL RECOVERED DIVIDED BY TOTAL LENGTH OF STRATUM AND EXPRESSED AS A PERCENTAGE.

STRATA ROCK QUALITY DESIGNATION (SRQD) - A MEASURE OF ROCK QUALITY DESCRIBED BY TOTAL LENGTH OF ROCK SEGMENTS WITHIN A STRATUM EQUAL TO OR GREATER THAN 4 INCHES DIVIDED BY THE TOTAL LENGTH OF STRATA AND EXPRESSED AS A PERCENTAGE.

TOPSOIL (TS.) - SURFACE SOILS USUALLY CONTAINING ORGANIC MATTER.

BENCH MARK: BL-101 (N:720429 E:1795435)

ELEVATION: 476.56 FEET

NOTES:

FIAD = FILLED IMMEDIATELY AFTER DRILLING

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NORTH CAROLINA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
GEOTECHNICAL ENGINEERING UNIT

SUBSURFACE INVESTIGATION

SUPPLEMENTAL LEGEND, GEOLOGICAL STRENGTH INDEX (GSI) TABLES FROM AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS

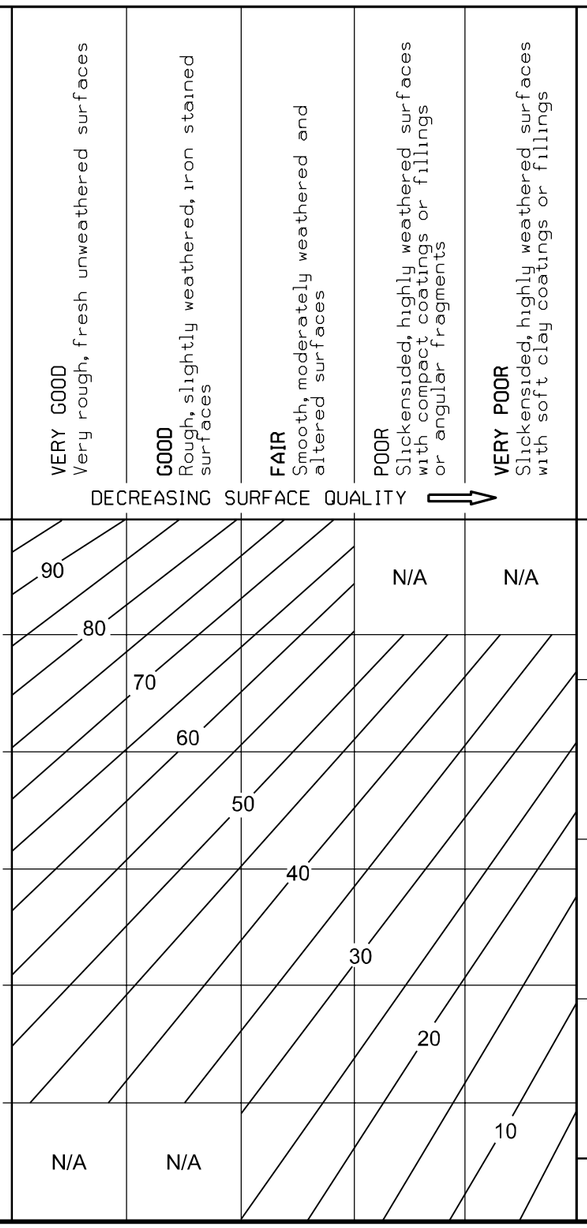
AASHTO LRFD Figure 10.4.6.4-1 — Determination of GSI for Jointed Rock Mass (Marinos and Hoek, 2000)

GEOLOGICAL STRENGTH INDEX (GSI) FOR JOINTED ROCKS (Hoek and Marinos, 2000)

From the lithology, structure and surface conditions of the discontinuities, estimate the average value of GSI. Do not try to be too precise. Quoting a range from 33 to 37 is more realistic than stating that GSI = 35. Note that the table does not apply to structurally controlled failures. Where weak planar structural planes are present in an unfavorable orientation with respect to the excavation face, these will dominate the rock mass behaviour. The shear strength of surfaces in rocks that are prone to deterioration as a result of changes in moisture content will be reduced if water is present. When working with rocks in the fair to very poor categories, a shift to the right may be made for wet conditions. Water pressure is dealt with by effective stress analysis.

STRUCTURE

- INTACT OR MASSIVE - intact rock specimens or massive in situ rock with few widely spaced discontinuities
- BLOCKY - well interlocked undisturbed rock mass consisting of cubical blocks formed by three intersecting discontinuity sets
- VERY BLOCKY - interlocked, partially disturbed mass with multi-faceted angular blocks formed by 4 or more joint sets
- BLOCKY/DISTURBED/SEAMY - folded with angular blocks formed by many intersecting discontinuity sets. Persistence of bedding planes or schistosity
- DISINTEGRATED - poorly interlocked, heavily broken rock mass with mixture of angular and rounded rock pieces
- LAMINATED/SHEARED - Lack of blockiness due to close spacing of weak schistosity or shear planes

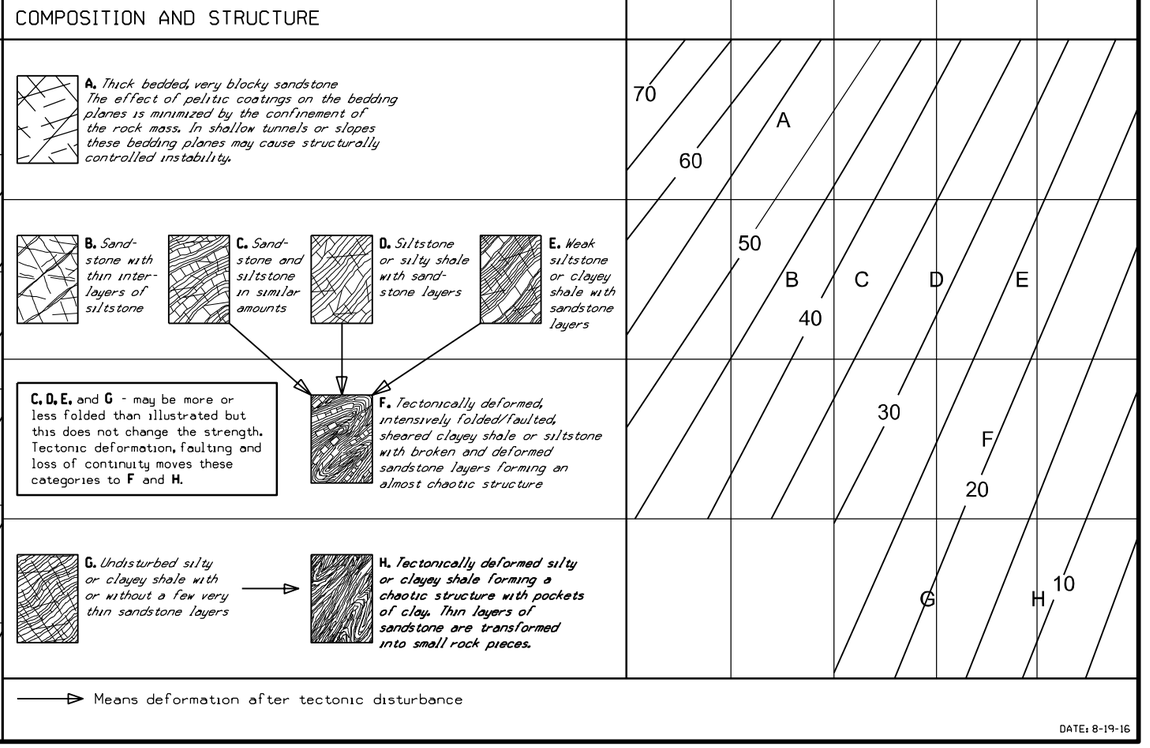


AASHTO LRFD Figure 10.4.6.4-2 — Determination of GSI for Tectonically Deformed Heterogeneous Rock Masses (Marinos and Hoek, 2000)

GSI FOR HETEROGENEOUS ROCK MASSES SUCH AS FLYSCH (Marinos. P and Hoek E., 2000)

From a description of the lithology, structure and surface conditions (particularly of the bedding planes), choose a box in the chart. Locate the position in the box that corresponds to the condition of the discontinuities and estimate the average value of GSI from the contours. Do not attempt to be too precise. Quoting a range from 33 to 37 is more realistic than giving GSI = 35. Note that the Hoek-Brown criterion does not apply to structurally controlled failures. Where unfavourably oriented continuous weak planar discontinuities are present, these will dominate the behaviour of the rock mass. The strength of some rock masses is reduced by the presence of groundwater and this can be allowed for by a slight shift to the right in the columns for fair, poor and very poor conditions. Water pressure does not change the value of GSI and it is dealt with by using effective stress analysis.

COMPOSITION AND STRUCTURE



DATE: 8-19-16

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PROJECT REFERENCE NO.	SHEET NO.
BP8.R026	3

SITE PLAN

0 50 100
FEET

NAD 83 NC GRID NA 2011

13

15

DB 2712 PG 2319
PB 165 PG 97

BEGIN PROJECT BP8.R026
-L- STA. 11+50.00

IS MTL BUS

TOWN OF FRANKLINVILLE
DB 2024 PG 983
PB 81PG 37
20' SEWER LINE EASEMENT

8" PVC SS

S 88°43'46" E
220.49'

EIP

S 51°01'10" W
73.46'

WOODS

DB 2726 PG 2364
PB 25 PG 60

END PROJECT BP8.R026
-L- STA. 16+50.00

END CONSTRUCTION
-L- STA. 16+70.00

GRAVEL

PM

REBAR & CAP
R/W MON

FRANKLINVILLE CITY LIMITS

GRASS

L_1215LT

TO NC64/NC49

HIGH WATER MARK=472.0'
⊕ BY FRIS MAPPING

REBAR

EIP(BENT)

FAITH ROCK ROAD SR 2207

EB1-A

EB2-A

BRIDGE #176

EB1-B

EB2-B

6" PVC

RIP RAP

L_1350RT

CONC
R/W MON

WOODS

TO FRANKLINVILLE

BEGIN CONSTRUCTION
-L- STA. 11+22.76

S 33°44'18" W
66.00'

UNNAMED CREEK

TOWN OF
FRANKLINVILLE
DB 2024 PG 983
PB 81 PG 37
20' SEWER LINE
EASEMENT

FRANKLINVILLE CITY LIMITS

S 86°45'21" W
1252.71'

DB 2210 PG 848
PB 83 PG 99

BM#1
N:721128 E:1795195
BL STATION 11+86.00 187' RIGHT
RRS IN 15" GUM TREE
477.04'

SKEW = 75 DEGREES

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5/14/99

PROJECT REFERENCE NO.	SHEET NO.
BP8.R026	4
ROADWAY DESIGN ENGINEER	HYDRAULICS ENGINEER

INCOMPLETE PLANS
DO NOT USE FOR R/W ACQUISITION

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

-L-

END BENT NO.1
(STA.14+15.51 -L-)

END BENT NO.2
(STA.14+86.16 -L-)

SS-2

EB1-B
14+30
12 FT RT

EB2-B
14+86
13 FT RT

490
485
480
475
470
465
460
455
450

GROUND SURFACE

ROADWAY EMBANKMENT
SOFT TO MEDIUM STIFF, MOIST, ORANGE-RED, CLAY (A-7-6) WITH SOME FINE TO COARSE SAND AND TRACE GRAVEL AND ROOTS

13
7
12
100/0.9
60/0.1
BT, DRY

6
4
7
5
03/23
100/0.7
100/0.3
BT

WEATHERED ROCK
TAN-GRAY, META-VOLCANIC

REC = 98%
RQD = 70%
GSI = 55-60

WATER SURFACE ELEV. 466.4 FEET (09/21/2022, BASED ON PROJECT BSR DATED ON 1/31/2024)

CRYSTALLINE ROCK
TAN-GRAY, MODERATELY TO SLIGHTLY WEATHERED, VERY CLOSELY TO MODERATELY CLOSELY FRACTURED, MODERATELY HARD TO HARD, META-VOLCANIC

(A) ROADWAY EMBANKMENT STIFF TO MEDIUM STIFF, MOIST, TAN-GRAY TO RED-ORANGE, FINE TO COARSE SANDY CLAY (A-6) WITH TRACE GRAVEL

(B) ALLUVIAL MEDIUM DENSE, MOIST, RED-ORANGE AND GRAY, CLAYEY GRAVEL (A-1-B)

(C) ALLUVIAL LOOSE, WET, ORANGE-BROWN, CLAYEY, FINE TO COARSE SAND (A-2-6) WITH TRACE GRAVEL

-GROUND LINE DRAWN FROM PROVIDED .TIN FILE ALONG CL (-L-)

-INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE PROFILE

SKEW = 75 DEGREES

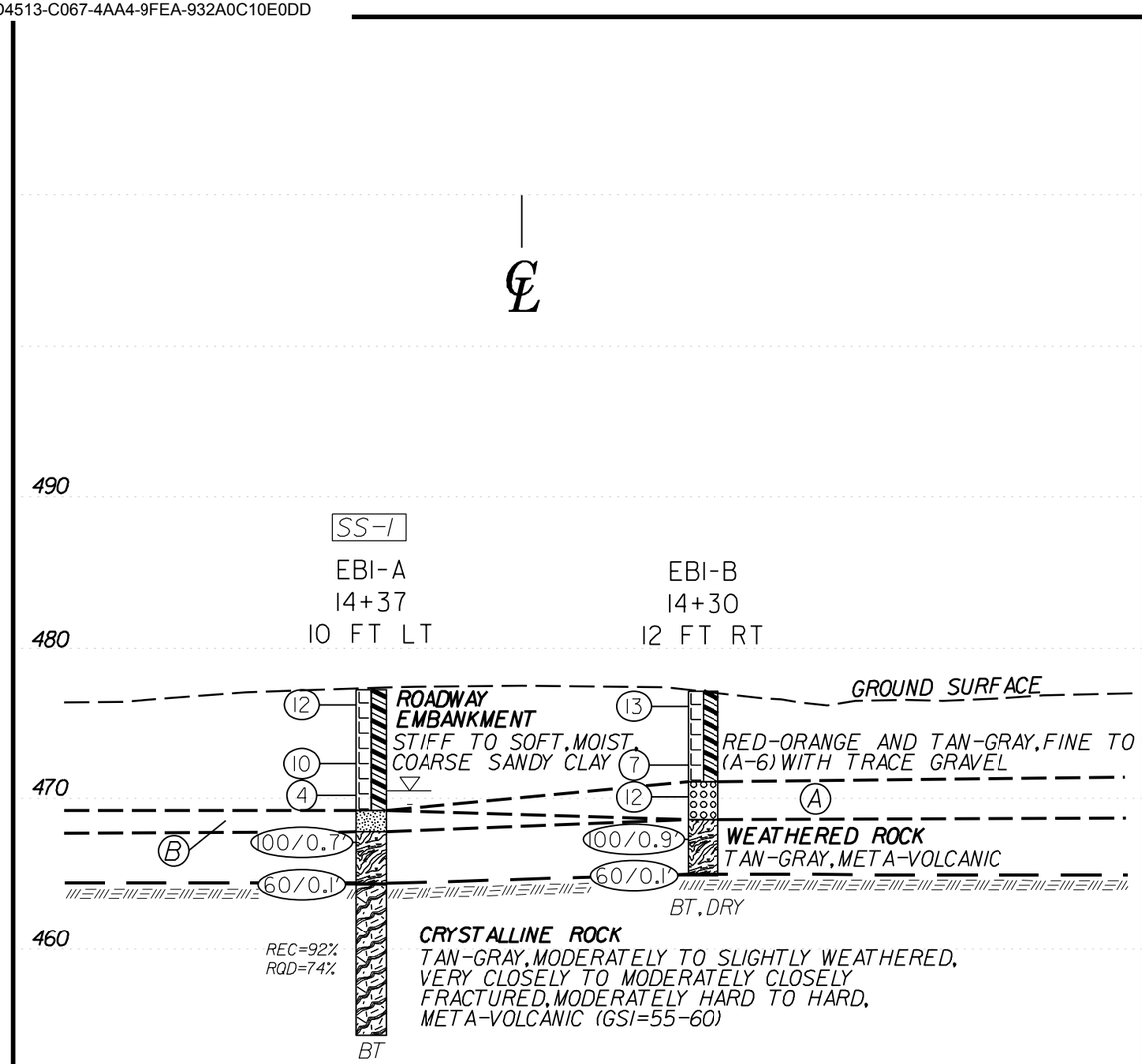
SCALE:
1" = 10' HORIZONTAL, 1" = 5' VERTICAL
VE = 2X

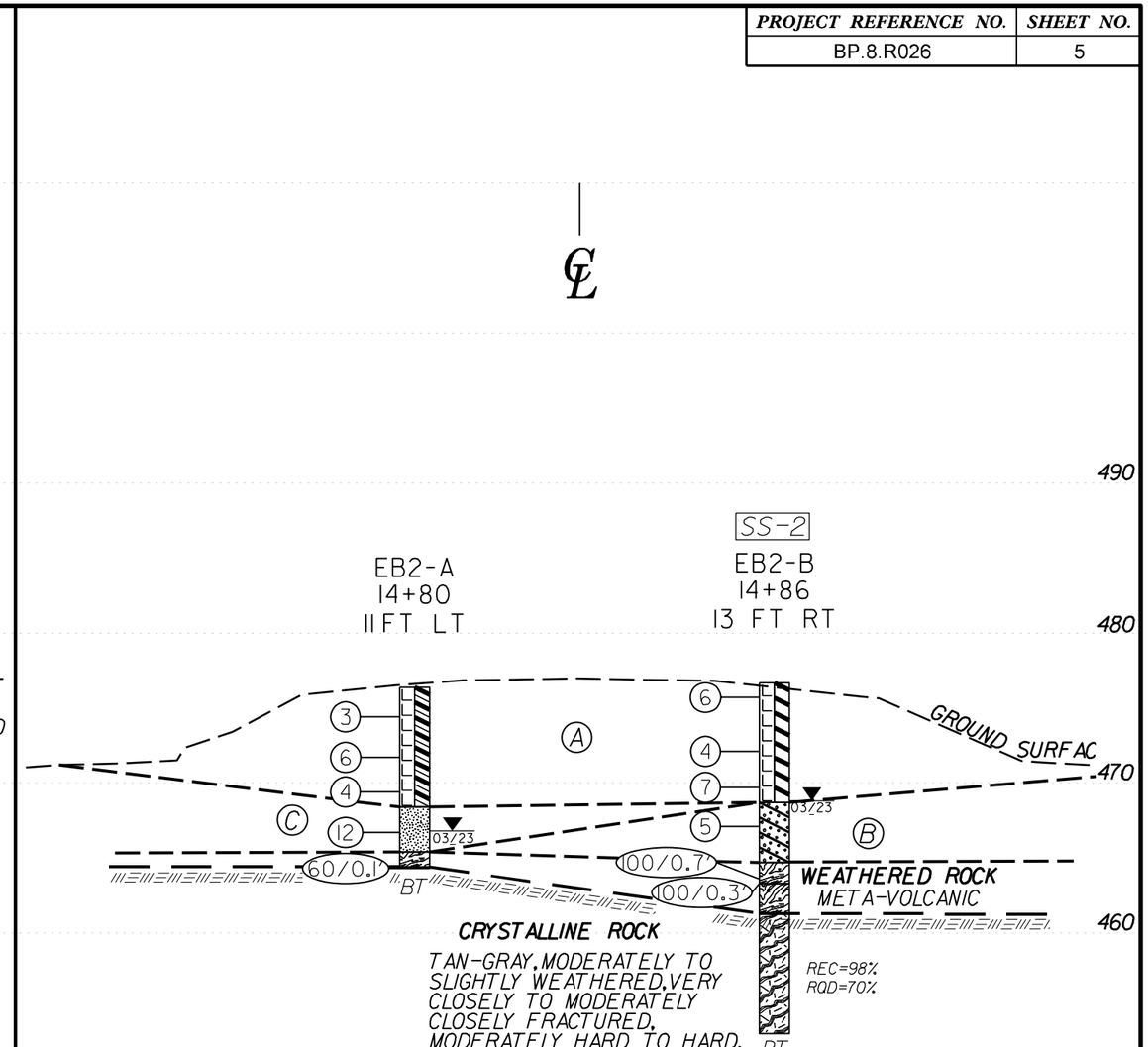
13+50 14+00 14+50 15+00 15+50 16+00

2/10/2025
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PROJECT REFERENCE NO.	SHEET NO.
BP.8.R026	5





Ⓐ **ALLUVIAL** MEDIUM DENSE, MOIST, RED-ORANGE AND GRAY, CLAYEY GRAVEL (A-1-B)

Ⓑ **RESIDUAL** MEDIUM STIFF TO HARD, MOIST, ORANGE-BROWN-GRAY, SANDY SILT (A-4), SAPROLITIC

- GROUND LINE CROSS SECTION DRAWN FROM PROVIDED .TIN FILE ALONG LINE OF BENT, SKEW OF 60 DEGREES.
- INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION.

14+15.51
-L-

SKEW = 75 DEGREES

HORIZ. SCALE (FEET) 0 10 20

VE = 1

CROSS-SECTION AT END BENT 1

Ⓐ **ROADWAY EMBANKMENT** SOFT TO MEDIUM STIFF, MOIST, ORANGE-RED AND BROWN-RED, SANDY CLAY (A-6) AND CLAY (A-7-6) WITH SOME FINE TO COARSE SAND AND TRACE GRAVEL AND ROOTS

Ⓑ **ALLUVIAL** LOOSE, WET, ORANGE-BROWN, CLAYEY, FINE TO COARSE SAND (A-2-6) WITH TRACE GRAVEL

Ⓒ **RESIDUAL** STIFF, MOIST, TAN-ORANGE, FINE SANDY, CLAYEY SILT (A-4), SAPROLITIC

- GROUND LINE CROSS SECTION DRAWN FROM PROVIDED .TIN FILE ALONG LINE OF BENT, SKEW OF 60 DEGREES.
- INFERRED STRATIGRAPHY IS DRAWN THROUGH THE BORINGS WITH BOTH PROJECTED ONTO THE CROSS SECTION.

14+86.16
-L-

SKEW = 75 DEGREES

HORIZ. SCALE (FEET) 0 10 20

VE = 1

CROSS-SECTION AT END BENT 2

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GEOTECHNICAL BORING REPORT
BORE LOG

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** J. Howard	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 14+37	**OFFSET** 10 ft LT	**ALIGNMENT** -L-	**0 HR.** 6.7
COLLAR ELEV. 477.2 ft	**TOTAL DEPTH** 22.9 ft	**NORTHING** 720,954	**EASTING** 1,795,089	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE BRI5184 CME-45C 87% 03/30/2022		DRILL METHOD SPT Core Boring		HAMMER TYPE Automatic
DRILLER J. Bradford	**START DATE** 03/22/23	**COMP. DATE** 03/22/23	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) DEPTH (ft)
480										
	477.2	0.0	4	5	7	12		M		477.2 GROUND SURFACE 0.0 **ROADWAY EMBANKMENT** Stiff to soft, moist, red-orange, fine to coarse sandy, CLAY (A-6) with trace gravel
475	473.3	3.9	8	6	4	10	SS-1	M		
	471.2	6.0	2	2	2	4		M		
470	468.3	8.9	6	25	75/0.2	100/0.7		M		469.2 8.0 **RESIDUAL** 467.8 9.4 Medium stiff to hard, moist, orange-brown-gray, sandy SILT (A-4), saprolitic
465	464.4	12.8	60/0.1			60/0.1				464.4 12.8 **WEATHERED ROCK** 464.3 12.9 Tan-gray, META-VOLCANIC rock
460										**CRYSTALLINE ROCK** Tan-gray, moderately to slightly weathered, very closely to moderately closely fractured, moderately hard to hard, META-VOLCANIC rock (REC=92%, RQD=74%, GSI=55-60)
455										454.3 22.9 Boring Terminated at Elevation 454.3 ft in Crystalline Rock: META-VOLCANIC rock

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** J. Howard	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. EB1-A	**STATION** 14+37	**OFFSET** 10 ft LT	**ALIGNMENT** -L-	**0 HR.** 6.7
COLLAR ELEV. 477.2 ft	**TOTAL DEPTH** 22.9 ft	**NORTHING** 720,954	**EASTING** 1,795,089	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE BRI5184 CME-45C 87% 03/30/2022		DRILL METHOD SPT Core Boring		HAMMER TYPE Automatic
DRILLER J. Bradford	**START DATE** 03/22/23	**COMP. DATE** 03/22/23	**SURFACE WATER DEPTH** N/A	
CORE SIZE NQ	**TOTAL RUN** 10.0 ft			

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
464.3											Begin Coring @ 12.9 ft
	464.3	12.9	5.0	0:45 1:30 4:00 2:45 2:45	(4.4) 88%	(2.8) 56%		(9.2) 92%	(7.4) 74%	464.3 12.9 **CRYSTALLINE ROCK** Tan-gray, moderately to slightly weathered, very closely to moderately closely fractured, moderately hard to hard, META-VOLCANIC (GSI=55-60) (4 joints at 15°, 6 joints at 45°, 5 joints at 60°, 1 joint at 80°, all with trace iron staining)
460	459.3	17.9	5.0	2:00 1:30 1:45 1:45 2:00	(4.8) 96%	(4.6) 92%					
455	454.3	22.9								454.3 22.9 Boring Terminated at Elevation 454.3 ft in Crystalline Rock: META-VOLCANIC rock	

NCDOT BORE DOUBLE BP8R026_GEO_BRDG_AS DRILLED BORINGS.GPJ NC_DOT.GDT 2/10/25

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PROJECT REFERENCE NO.	SHEET NO.
BP8.R026	7

CORE PHOTOGRAPHS

EB1-A

BOX 1 of 1: 12.9-22.9 FEET



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GEOTECHNICAL BORING REPORT
BORE LOG

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** J. Howard	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. EB1-B	**STATION** 14+30	**OFFSET** 12 ft RT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 477.1 ft	**TOTAL DEPTH** 12.2 ft	**NORTHING** 720,963	**EASTING** 1,795,110	**24 HR.** Dry
DRILL RIG/HAMMER EFF./DATE BRI5184 CME-45C 87% 03/30/2022		DRILL METHOD H.S. Augers	HAMMER TYPE Automatic	
DRILLER J. Bradford	**START DATE** 03/20/23	**COMP. DATE** 03/20/23	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
										ELEV. (ft) DEPTH (ft)
480										
	477.1	0.0	4	8	5	13		M		477.1 GROUND SURFACE 0.0
475										**ROADWAY EMBANKMENT**
	473.2	3.9	2	3	4	7		M		Stiff to medium stiff, moist, tan-gray to red-orange, fine to coarse sandy CLAY (A-6) with trace gravel
	471.1	6.0	7	7	5	12		M		471.1 6.0
470										**ALLUVIAL**
	468.2	8.9	34	66/0.4		100/0.9		M		Medium dense, moist, red-orange and gray, clayey GRAVEL (A-1-b) 468.6 8.5
										WEATHERED ROCK
465	465.0	12.1	60/0.1			60/0.1				Tan-gray, META-VOLCANIC rock 465.0 12.1
										464.9 **CRYSTALLINE ROCK** 12.2
										Gray, META-VOLCANIC rock
										Boring Terminated with Standard Penetration Test Refusal at Elevation 464.9 ft in Crystalline Rock: META-VOLCANIC rock

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** J. Howard	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. EB2-A	**STATION** 14+80	**OFFSET** 11 ft LT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 476.4 ft	**TOTAL DEPTH** 12.1 ft	**NORTHING** 720,987	**EASTING** 1,795,061	**24 HR.** 9.6
DRILL RIG/HAMMER EFF./DATE BRI5184 CME-45C 87% 03/30/2022		DRILL METHOD H.S. Augers	HAMMER TYPE Automatic	
DRILLER J. Bradford	**START DATE** 03/20/23	**COMP. DATE** 03/20/23	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
480										
	475.4	1.0	1	1	2	3		M		476.4 GROUND SURFACE 0.0
475										**ROADWAY EMBANKMENT**
	472.7	3.7	3	3	3	6		M		Soft to medium stiff, moist, brown-red, fine sandy, silty CLAY (A-6)
	470.4	6.0	1	2	2	4		M		468.4 8.0
470										**RESIDUAL**
	467.7	8.7	5	7	5	12		M		Stiff, moist, tan-orange, fine sandy, clayey SILT (A-4), saprolitic 465.4 11.0
465	464.4	12.0	60/0.1			60/0.1				464.4 **WEATHERED ROCK** 12.0
										META-VOLCANIC rock 464.3 **CRYSTALLINE ROCK** 12.1
										META-VOLCANIC rock
										Boring Terminated with Standard Penetration Test Refusal at Elevation 464.3 ft in Crystalline Rock: META-VOLCANIC rock

NCDOT BORE DOUBLE BP8R026_GEO_BRDG_AS DRILLED BORINGS.GPJ NC_DOT.GDT 2/10/25

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GEOTECHNICAL BORING REPORT
BORE LOG

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** J. Howard	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 14+86	**OFFSET** 13 ft RT	**ALIGNMENT** -L-	**0 HR.** 7.9
COLLAR ELEV. 476.7 ft	**TOTAL DEPTH** 23.4 ft	**NORTHING** 721,007	**EASTING** 1,795,076	**24 HR.** 7.9
DRILL RIG/HAMMER EFF./DATE BRI5184 CME-45C 87% 03/30/2022		DRILL METHOD SPT Core Boring	HAMMER TYPE Automatic	
DRILLER J. Bradford	**START DATE** 03/20/23	**COMP. DATE** 03/21/23	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
480											
	476.7	0.0								476.7 GROUND SURFACE	0.0
475			3	2	4	6		M		**ROADWAY EMBANKMENT** Soft to medium stiff, moist, orange-red, CLAY (A-7-6) with some fine to coarse sand and trace gravel and roots	
	473.1	3.6	1	2	2	4	SS-2	M			
470	470.7	6.0	3	3	4	7		M			
	468.1	8.6	4	3	2	5		W		468.7 **ALLUVIAL** Loose, wet, orange-brown, clayey, fine to coarse SAND (A-2-6) with trace gravel	8.0
465	464.3	12.4								464.7 **WEATHERED ROCK** Tan-gray, META-VOLCANIC rock	12.0
	463.6	13.1	37	63/0.2		100/0.7				463.3 **WEATHERED ROCK** No Recovery - META-VOLCANIC rock	13.4
460			100/0.3			100/0.3				461.3 **CRYSTALLINE ROCK** Tan-gray, moderately to slightly weathered, very closely to moderately closely fractured, moderately hard to hard, META-VOLCANIC rock (REC=98%, RQD=70%, GSI=55-60)	15.4
455										453.3 Boring Terminated at Elevation 453.3 ft in Crystalline Rock: META-VOLCANIC rock	23.4

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** J. Howard	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. EB2-B	**STATION** 14+86	**OFFSET** 13 ft RT	**ALIGNMENT** -L-	**0 HR.** 7.9
COLLAR ELEV. 476.7 ft	**TOTAL DEPTH** 23.4 ft	**NORTHING** 721,007	**EASTING** 1,795,076	**24 HR.** 7.9
DRILL RIG/HAMMER EFF./DATE BRI5184 CME-45C 87% 03/30/2022		DRILL METHOD SPT Core Boring	HAMMER TYPE Automatic	
DRILLER J. Bradford	**START DATE** 03/20/23	**COMP. DATE** 03/21/23	**SURFACE WATER DEPTH** N/A	
CORE SIZE NQ	**TOTAL RUN** 10.0 ft			

ELEV (ft)	RUN ELEV (ft)	DEPTH (ft)	RUN (ft)	DRILL RATE (Min/ft)	RUN REC. (ft) %	RUN RQD (ft) %	SAMP. NO.	STRATA REC. (ft) %	STRATA RQD (ft) %	LOG	DESCRIPTION AND REMARKS
463.3											Begin Coring @ 13.4 ft
	463.3	13.4	5.0	1:00 0:45 2:15 3:45 10:30	(3.0) 60%	(1.9) 38%		(0.0) 0%	(0.0) 0%	463.3 461.3	**WEATHERED ROCK** 13.4 No Recovery - META-VOLCANIC rock 15.4
460								(7.8) 98%	(5.6) 70%		**CRYSTALLINE ROCK** Tan-gray, moderately to slightly weathered, very closely to moderately closely fractured, moderately hard to hard, META-VOLCANIC rock (GSI=55-60) (3 joints at 15°, 3 joints at 45°, 8 joints at 60°, all with trace iron staining)
	458.3	18.4	5.0	6:00 4:15 4:00 4:30 7:30	(4.8) 96%	(3.7) 74%					
455	453.3	23.4								453.3	23.4 Boring Terminated at Elevation 453.3 ft in Crystalline Rock: META-VOLCANIC rock

NCDOT BORE DOUBLE BP8R026_GEO_BRDG_AS DRILLED BORINGS.GPJ NC_DOT.GDT 2/10/25

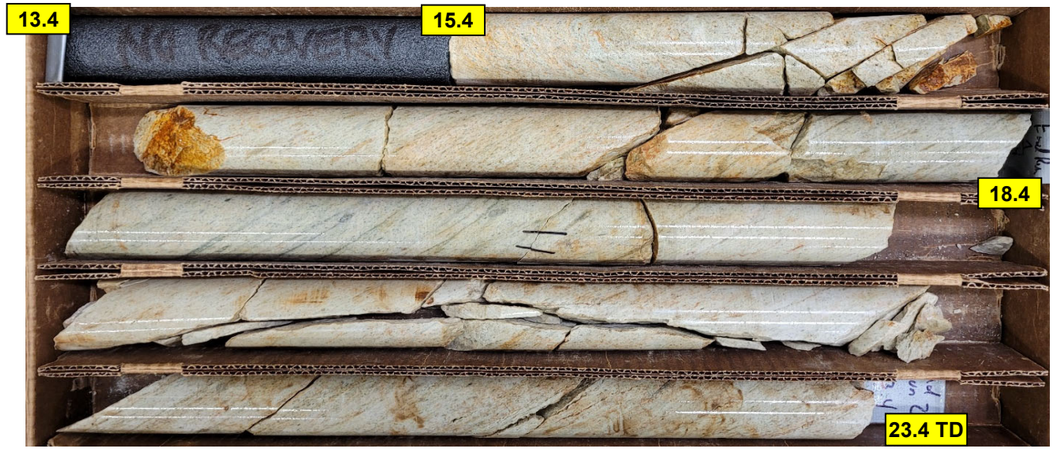
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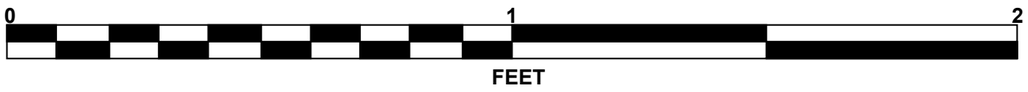
PROJECT REFERENCE NO.	SHEET NO.
BP8.R026	10

CORE PHOTOGRAPHS

EB2-B

BOX 1 of 1: 13.4 - 23.4 FEET





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GEOTECHNICAL BORING REPORT
BORE LOG

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** C.T. Tang	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. L_1215LT	**STATION** 12+15	**OFFSET** 30 ft LT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 495.4 ft	**TOTAL DEPTH** 2.0 ft	**NORTHING** 720,770	**EASTING** 1,795,210	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE N/A		DRILL METHOD Hand Auger	HAMMER TYPE Automatic	
DRILLER C.T. Tang	**START DATE** 12/21/23	**COMP. DATE** 12/21/23	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION ELEV. (ft)	DEPTH (ft)
500											
										495.4 GROUND SURFACE	0.0
495										**RESIDUAL**	
										493.4 Stiff, moist, red and tan, sandy SILT (A-4), with Some Roots	2.0
										Boring Terminated at Elevation 493.4 ft in Residual Sandy Silt (A-4)	

WBS BP8.R026.1	**TIP** N/A	**COUNTY** RANDOLPH	**GEOLOGIST** C.T. Tang	
SITE DESCRIPTION BP8.R026 - Bridge No. 176 over Creek on SR 2207 (Faith Rock Road)				**GROUND WTR (ft)**
BORING NO. L_1350RT	**STATION** 13+50	**OFFSET** 35 ft RT	**ALIGNMENT** -L-	**0 HR.** Dry
COLLAR ELEV. 490.5 ft	**TOTAL DEPTH** 3.0 ft	**NORTHING** 720,915	**EASTING** 1,795,178	**24 HR.** FIAD
DRILL RIG/HAMMER EFF./DATE N/A		DRILL METHOD Hand Auger	HAMMER TYPE Automatic	
DRILLER C.T. Tang	**START DATE** 12/21/23	**COMP. DATE** 12/21/23	**SURFACE WATER DEPTH** N/A	

ELEV (ft)	DRIVE ELEV (ft)	DEPTH (ft)	BLOW COUNT 0.5ft	0.5ft	0.5ft	BLOWS PER FOOT 0 25 50 75 100	SAMP. NO.	MOI	LOG	SOIL AND ROCK DESCRIPTION
495										
										490.5 GROUND SURFACE 0.0
490							S-1			**RESIDUAL**
										487.5 Stiff, moist to wet, red and tan, sandy SILT (A-4), saprolitic, with Some Roots in upper 1 foot 3.0
										Boring Terminated at Elevation 487.5 ft in Residual Sandy Silt (A-4)

NCDOT BORE DOUBLE BP8R026_GEO_BRDG_AS DRILLED BORINGS.GPJ NC_DOT.GDT 2/10/25

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PROJECT REFERENCE NO.	SHEET NO.
BP8.R026	13

LABORATORY TESTS PERFORMED ON 12-29-2023

LABORATORY TEST RESULTS

SAMPLE NO.	BORING	STATION	OFFSET	ALIGNMENT	NORTHING	EASTING	DEPTH INTERVAL	AASHTO CLASS.	L.L.	P.I.	% BY WEIGHT				% BY PASSING SEIVES			% MOISTURE	% ORGANIC
											GRAVEL	SAND	SILT	CLAY	10	40	200		
SS-1	EB1-A	14+37	10 FT LT	720,954	1,795,089	-L-	3.9'-5.4'	A-6	34	14	14.4	38.9	27.5	19.2	78.5	66.1	46.7	-*	-
SS-2	EB2-B	14+86	13 FT RT	721,007	1,795,076	-L-	3.6'-5.1'	A-7-6	43	16	11.5	14.5	39.0	35.0	86.6	82.5	74.0	-*	-
S-1	L_1350RT	13+50	35 FT RT	720,915	1,795,178	-L-	1.5'-2.5'	A-4	29	7	0.0	23.6	54.4	22.0	98.8	91.9	76.4	17.7	-

*Soil samples were tested 9 months after the samples were taken; thus, samples were dried out and natural moisture content was not tested.

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PROJECT REFERENCE NO.	SHEET NO.
BP8.R026	14

SITE PHOTOGRAPH



LOOKING WEST FROM EAST SIDE OF END BENT 1